An Adaptive Moving Finite Volume Scheme for Shallow Water Equations with Dry and Complex Topography

# Guoxian CHEN

gxchen@igpm.rwth-aachen.de

IGPM, RWTH Aachen, Germany.

Hyp2012, Padova, Italian, June 25, 2012

## 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

## Output State Numerical Examples

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

#### 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

Movie

Shallow water equations with bed slope and friction stress source terms

$$\begin{cases} h_t + \nabla \cdot (h\vec{u}) = 0, \\ h\vec{u} + \nabla \cdot (h\vec{u} \otimes \vec{u}) + \nabla \left(\frac{1}{2}gh^2\right) = -gh\nabla b - \tau_{\mathbf{b}}/\rho, \end{cases}$$
(1)

Manning friction model

$$\tau_b = (\tau_{bx}, \tau_{by}) = \rho C_f \vec{u} |\vec{u}| \tag{2}$$

where

$$C_f = gn^2/h^{1/3}$$

Still water equilibrium

$$\begin{cases} \vec{u} = 0, \\ w = h + b = \max(C, b) \end{cases}$$
(3)

water depth positivity

$$h \ge 0$$
 (4)

### 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

## 2 Numerical Scheme

### PDEs discretization

- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

- Perthame B, Simeoni C. A kinetic scheme for the Saint-Venant system with a source term. Calcolo 2001; 38(4):201–231.
- LeVeque R. Balancing source terms and flux gradients in high-resolution Godunov methods: the quasi-steady wave-propagation algorithm. Journal of Computational Physics 1998; 146(1):346–365.
- Zhou J, Causon D, Mingham C, Ingram D. The surface gradient method for the treatment of source terms in the shallow-water equations. Journal of Computational Physics 2001; 168(1):1–25.
- Noelle S, Xing Y, Shu C. High-order well-balanced finite volume WENO schemes for shallow water equation with moving water. Journal of Computational Physics 2007; 226(1):29–58.
- Noelle S, Pankratz N, Puppo G, Natvig J. Well-balanced finite volume schemes of arbitrary order of accuracy for shallow water flows. Journal of Computational Physics 2006; 213(2):474–499.
- Li J, Chen G. The generalized Riemann problem method for the shallow water equations with bottom topography. International Journal for Numerical Methods in Engineering 2006; 65(6):834–862.
- Audusse E, Bouchut F, Bristeau M, Klein R, Perthame B. A fast and stable well-balanced scheme with hydrostatic reconstruction for shallow water flows. *Journal of Scientific Computation* 2004; 25(6):2050–2065.
- Audusse E, Bristeau M. A well-balanced positivity preserving "second-order" scheme for shallow water flows on unstructured meshes. *Journal of Computational Physics* 2005; 206(1):311–333.
- Berthon C, Foucher F. Efficient well-balanced hydrostatic upwind schemes for shallow-water equations. Journal of Computational Physics 2012; .

o . . .

### Splitting scheme

$$\begin{cases} \vec{U}_{i} = \vec{U}_{i}^{n} - \frac{\Delta t}{|E_{i}|} \sum_{j=1}^{3} \vec{F}_{\vec{n}_{ij}}(\vec{U})_{ij}^{n} |l_{ij}| + \Delta t \vec{S}_{b}(\vec{U})_{i}^{n} \\ (hu)_{i}^{n+1} = \widetilde{\vec{u}}_{i} - \Delta t S_{f}(\vec{\vec{U}}_{i}). \end{cases}$$

$$(5)$$

 Li J, Chen G. The generalized Riemann problem method for the shallow water equations with bottom topography. International Journal for Numerical Methods in Engineering 2006; 65(6):834–862.

[2] Audusse E, Bristeau M. A well-balanced positivity preserving "second-order" scheme for shallow water flows on unstructured meshes. Journal of Computational Physics 2005; 206(1):311–333.

### STEP 1: Hydrostatic reconstruction

$$\begin{cases} b_{ij} = \max(b_{ij}^{L}, b_{ij}^{R}), \\ h_{ij}^{L,*} = \max(b_{ij}^{L} + h_{ij}^{L} - b_{ij}, 0), \\ h_{ij}^{R,*} = \max(b_{ij}^{R} + h_{ij}^{R} - b_{ij}, 0) \end{cases} \Rightarrow \begin{cases} \vec{U}_{ij}^{L} = (h_{ij}^{L,*}, h_{ij}^{L,*} \vec{u}_{ij}^{L}) \\ \vec{U}_{ij}^{R} = (h_{ij}^{R,*}, h_{ij}^{R,*} \vec{u}_{ij}^{R}) \end{cases}$$
(6)

## STEP 2: Flux evaluation

$$\vec{F}_{\vec{n}_{ij}}(\vec{U})_{ij}^{n} = \vec{F}_{\vec{n}_{ij}}(\vec{U}_{ij}^{L},\vec{U}_{ij}^{R}) + \frac{g}{2}((h_{ij}^{L})^{2} - (h_{ij}^{L,*})^{2})$$
(7)

STEP 3: Bed slop discretization

$$\vec{S}_{b}(\vec{U})_{i}^{n} = -\frac{1}{|E_{i}|} \sum_{j=1}^{3} \begin{pmatrix} 0 \\ g \frac{h_{ij}^{L} + \bar{h}_{i}}{2} \left( b_{ij}^{L} - b_{i} \right) |l_{ij}| \vec{n}_{ij} \end{pmatrix},$$
(8)

A partially implicit approach

$$\frac{(h\vec{u})^{n+1} - (\tilde{h}\tilde{\vec{u}})}{\Delta t} = -\frac{gn^2(h\vec{u})^{n+1}|\tilde{\vec{u}}|}{(\tilde{h})^{4/3}},\tag{9}$$

then

$$(h\vec{u})^{n+1} = \frac{(\tilde{h}\vec{\tilde{u}})}{1 + \Delta t g n^2 |\vec{\tilde{u}}| / (\tilde{h})^{4/3}} = \frac{(\tilde{h}\vec{\tilde{u}})(\tilde{h})^{4/3}}{(\tilde{h})^{4/3} + \Delta t g n^2 |\vec{\tilde{u}}|},$$
(10)

Well-balance, Positivity, Stable friction discretization.

### 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

## The need of Adaptive method

- Large variation of solution in the local domain.
- To see more detail information
- To capture the Wet/dry front



- Refinement method(h-method):
- Polynomial method(p-method):
- Moving mesh method (r-method):

# The history of moving mesh method

- Winslow's moving mesh method based on variational principle;
   A. Winslow, J. Comput. Phys., 1(1967), pp. 149-172.
- de Boor's equal-distribution principle;
   C. de Boor, Lecture notes in Math. 363 Springer-Verlag, Berlin,1973.
- Miller's moving FEM(MFE);
   K. Miller and R.N. Miller, SIAM J. Numer Anal., 18(1981), pp. 1019–1032.
- Weizhang Huang's moving mesh method based on parabolic PDE(MMPDE); W.Z. Huang, J. Comput. Phys., 171 (2001), pp. 753-775.; 204(2005), pp. 633-665.
- Harmonic map methods: R.Li, Tao. Tang and P.W.Zhang;
   R. Li, T. Tang and P.W. Zhang, J. Comput. Phys., 170(2001), pp. 562-588.; 177(2002), pp. 365-393.
- Variation principle method:H.Z.Tang and T.Tang H.Z. Tang and T. Tang, SIAM J. Numer. Anal., 41(2001), pp. 487-515.
- Tang H. Solution of the shallow-water equations using an adaptive moving mesh method. International Journal for Numerical Methods in Fluids 2004; 44(7):789–810.
- Chen G, Tang H, Zhang P. Second-order accurate Godunov scheme for multicomponent flows on moving triangular meshes. *Journal of Scientific Computing* 2008; 34(1):64–86.

...

- The Moving of meshes: A mesh PDE decide a mapping from the logical domain to the physical domain. Solving it by FEM/FVM will get a new position of the meshes of the physical domain.
- Monitor function: To control the density of grid.
- The interpolation of solution from old mesh to new mesh:: conservation, maximum principle, divergence free. Still water preserving, Positivity

From the variational principle,

$$ilde{E}(oldsymbol{x}) = rac{1}{2}\sum_{i=1}^d \int_{\Omega_l} ( ilde{
abla} oldsymbol{x}_i)^T G_i ilde{
abla} oldsymbol{x}_i \,\, doldsymbol{k},$$

we get the Euler-Lagrange equation

$$\mathbf{\tilde{\nabla}} \cdot (\mathbf{\omega}\mathbf{\tilde{\nabla}}\mathbf{\vec{x}}) = \mathbf{0}.$$

It gives a mapping from the logical domain to the physical domain.

[1] A. Winslow, J. Comput. Phys., 1(1967), pp. 149-172.

[2] H.Z. Tang and T. Tang, SIAM J. Numer. Anal., 41(2001), pp. 487-515.

[3] G.X. Chen, H.Z. Tang and P.W. Zhang, J. Sci. Comput., 34(2008), pp. 64-86.

Let the meshes on  $\Omega_l$  is  $\mathcal{T}_l$ , We choose the trial function space

$$V_h=ig\{v\in C^0(\Omega_l): v|_E\in P^1(E), \quad orall E\in \mathcal{T}_l, ext{and} v|_{\partial\Omega_l}=ec{x}^n|_{\partial\Omega_l}ig\},$$

and the test function space

$$V_{h,\mathbf{0}} = \big\{ v \in C^{\mathbf{0}}(\Omega_l) : v|_E \in P^1(E), \quad \forall E \in \mathcal{T}_l, \mathsf{and} v|_{\partial \Omega_l} = \mathbf{0} \big\}.$$

Find  $ec{x}_h(ec{\xi})\in V_h$  such that

$$\int_{\Omega_l}\omega ilde{
abla}ec{x}_h(ec{\xi})\cdot ilde{
abla}artual(ec{\xi})d\xi=0,\quad orall artual(ec{k})d\xi=0,$$

Let  $ec{x}_h(ec{\xi}) = \sum_{j=1}^{nd} ec{x}_i \varphi^i(ec{\xi})$ , then

$$\sum_{j=1}^{nd} ec{x}_j \int_{\Omega_l} \omega ilde{
abla} \varphi^j(ec{\xi}) \cdot ilde{
abla} \varphi^i(ec{\xi}) d\xi = 0, \quad i=1,2,\cdots,ndi,$$

let

$$a_{ij} = \int_{\Omega_l} \omega \tilde{\nabla} \phi^j(\vec{\xi}) \cdot \tilde{\nabla} \phi^i(\vec{\xi}) d\xi.$$

# Relax Jacobi iterations method



# To avoid the grid winding:CFL type condition

$$egin{array}{l} \max f( au) = au \ ext{s.t.} - \min\{|D_{ij}( au)|, 0\} < |E_{ij}^*| := rac{|E_i^{\lceil au
ceil}|}{\Lambda(d)}, \quad orall i, j, \end{array}$$

$$|E_i^{[
u+1]}| = |E_i^{[
u]}| + \sum_{j=1}^{\Lambda(d)} |D_{ij}| = \sum_{j=1}^{\Lambda(d)} (rac{1}{\Lambda(d)} |E_i^{[
u]}| + |D_{ij}|) > 0.$$



 $\Rightarrow$ 



[1] J.Q. Han and H.Z. Tang, J. Comput. Phys., 220(2007), pp. 791-812.

[2] G.X. Chen, H.Z. Tang and P.W. Zhang, J. Sci. Comput., 34(2008), pp. 64-86.

$$\left\{egin{array}{ll} |D_{ij}|&=-|D_{ji}|,\ |B_i^{[
u+1]}|&=|E_i^{[
u]}|+\sum_{j=1}^{\Lambda(d)}|D_{ij}|. \end{array}
ight.$$



$$egin{aligned} |E_i^{[ extsf{v}+1]}|\overline{u}_{E_i^{[ extsf{v}+1]}} &= |E_i^{[ extsf{v}]}|\overline{u}_{E_i^{[ extsf{v}]}} + \sum_{j=1}^{\Lambda(d)} \widetilde{F}(u_{ij}^L, u_{ij}^R), \ &\widetilde{F}(u_{ij}^L, u_{ij}^R) &= \max\{|D_{ij}|, 0\}u_{ij}^R + \min\{|D_{ij}|, 0\}u_{ij}^L, \ &\widetilde{F}(u_{ij}^L, u_{ij}^R) &= -\widetilde{F}(u_{ij}^R, u_{ij}^L) \end{aligned}$$

$$\sum_{i} |E_{i}^{[
u]}|\overline{u}_{E_{i}^{[
u]}}| = \sum_{i} |E_{i}^{[
u+1]}|\overline{u}_{E_{i}^{[
u+1]}}.$$
 $\widetilde{\mathcal{F}}(u_{ij}^{L}, u_{ij}^{R}) = \max\{|D_{ij}|, 0\}\mathcal{S}(u_{ij}^{R}) + \min\{|D_{ij}|, 0\}\mathcal{S}(u_{ij}^{L}).$ 

 $\Rightarrow$ 

$$\begin{split} |E_{i}^{[\nu+1]}|\overline{h}_{E_{i}^{[\nu+1]}} &= |E_{i}^{[\nu]}|\overline{h}_{E_{i}^{[\nu]}} + \sum_{j=1}^{\Lambda(d)} (\max\{|D_{ij}|, 0\}h_{ij}^{R} + \min\{|D_{ij}|, 0\}h_{ij}^{L}), \\ &= \sum_{j=1}^{\Lambda(d)} \frac{|E_{i}^{[\nu]}|}{\Lambda(d)}\overline{h}_{ij}^{L} + \sum_{j=1}^{\Lambda(d)} (\max\{|D_{ij}|, 0\}h_{ij}^{R} + \min\{|D_{ij}|, 0\}h_{ij}^{L}), \\ &= \sum_{j=1}^{\Lambda(d)} (\frac{|E_{i}^{[\nu]}|}{\Lambda(d)} + \min\{|D_{ij}|, 0\})h_{ij}^{L} + \sum_{j=1}^{\Lambda(d)} \max\{|D_{ij}|, 0\}h_{ij}^{R}, \\ &\geq 0 \end{split}$$

$$\begin{split} |E_i^{[\nu+1]}|\overline{q}_{E_i^{[\nu+1]}} &= |E_i^{[\nu]}|\overline{q}_{E_i^{[\nu]}} + \sum_{j=1}^{\Lambda(d)} (\max\{|D_{ij}|, 0\}q_{ij}^R + \min\{|D_{ij}|, 0\}q_{ij}^L), \\ &= |E_i^{[\nu]}|C + \sum_{j=1}^{\Lambda(d)} (\max\{|D_{ij}|, 0\}C + \min\{|D_{ij}|, 0\}C), \\ &= (|E_i^{[\nu]}| + \sum_{j=1}^{\Lambda(d)} (\max\{|D_{ij}|, 0\} + \min\{|D_{ij}|, 0\}))C, \\ &\overline{q}_{E_i^{[\nu+1]}} = C \end{split}$$

 $\Rightarrow$ 

### 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

### Step 1: Adaptive moving of triangular meshes

A1 Adjust the cells  $\vec{x}^{[\nu]}$  to the new position  $\vec{x}^{[\nu+1]}$ . A2 Update the conservative flow variables  $\vec{U}_i^{[\nu+1]}$ . A3 Stop criterion:  $\left|\vec{x}_i^{[\nu+1]} - \vec{x}_i^{[\nu]}\right| < \epsilon$  .or.  $\nu = 5$ Step 2: Update  $\vec{U}_i^{n+1}$  over the new fixed meshes  $\vec{x}_i^{n+1}$ :. B1 Update the solution  $\widetilde{\vec{U}}_i$  without the friction . B2 Update the solution  $\vec{U}_i^{n+1}$  for friction term .

### 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

,

Arc-Length type Monitor function

$$\omega_i = \sqrt{1 + \alpha \tilde{\omega}_i^2(\beta, h+b)}$$

$$\begin{split} \tilde{w}_i(\beta, h+b) &=: \min\left\{1, \left|\nabla_{\xi}(h+b)\right|_i \middle/ \beta \max_i \left|\nabla_{\xi}(h+b)\right|_i\right\}\\ \alpha &= 40.0 \quad \beta = 0.25 \end{split}$$

## 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

### Bottom topograph

$$B(x,y) = 0.8 \exp\left\{-50\left[(x-0.5)^2 + (y-0.5)^2\right]
ight\}, (x,y) \in [0,1] \times [0,1],$$
 (11)

Initial condition

$$\begin{cases} h(x, y) = 1 - B(x, y), \\ \vec{u}(x, y, 0) = 0. \end{cases}$$
(12)

Results at time t = 1.7s

Algorithm	h+b-1	u	v
Fixed meshes	2.00E-15	4.06E-14	4.44E-14
Moving meshes	7.22E-15	1.39E-14	1.77E-14

## 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

### Output State Numerical Examples

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

### Bottom topograph

$$B(x,y) = 0.8 \exp\left\{-5(x-0.9)^2 + 50(y-0.5)^2\right\}, (x,y) \in [0,2] \times [0,1].$$
(13)

Still Initial condition

$$h(x,y) = \begin{cases} 1+\varepsilon - B(x,y), & \text{if } x \in [0.05, 0.15] \text{ m}, \\ 1-B(x,y), & \text{Otherwise,} \end{cases}$$
(14)

where  $\epsilon=0.01$ 

# Numerical Results at t = 0.24s and 0.48s



## 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

# Analytical transcritical flow with shock.

Initial and Boundary conditions

$$\mathsf{IC}: q = 2m^2/s; h(x) = \max(h_{st}(100) + B(100) - B(x), 0.0)$$

BC : upstream : q = 2; downstream :  $h_{st}(100)$ 



## 2 Numerical Scheme

- PDEs discretization
- Adaptive Moving mesh methods
- Solution algorithm

### Output State Numerical Examples

- Still water flow over a smooth bed
- Still water with a small perturbation
- Analytical transcritical flow with shock
- Numerical dam-break inundation with closed boundary

## Close Channel dam-break.

Initial and Boundary conditions

$$IC: h(x) = 1.875 \quad x < 16$$

BC : reflection boundary condition





# Close Channel dam-break.



movie

Movie

- Prof. Sebastian Noelle, RWTH Aachen, Germany.
- Prof. Huazhong Tang, Peking University, China.
- Dr. Feng Zhou, Peking University, China
- Funding: DFG Grant of Germany NO361/3-1 and No361/3-2; National Natural Science Foundation of China (No.11001211, 51178359);

Thanks you